

Land Capability Assessment

Draper Road, Heyfield



**PLANNING AND ENVIRONMENT ACT 1987
WELLINGTON PLANNING SCHEME**

This is the plan referred to in Clause 43.04 - Schedule 11 of Development Plan Overlay and has been approved by Wellington Shire Council

DATE: 25 September 2023


**SIGNED: Caragh Button
STRATEGIC PLANNER**

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Distribution

Land Capability Assessment



Draper Road, Heyfield

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Accreditation Land Capability Assessment for On-site Wastewater Management Certificate CET, 2015
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| Edition | Description | Date |
|---------|-------------|-----------|
| Rev0 | Version 1 | 6/03/2023 |

1. SUMMARY

The following summary table should be read in conjunction with the entire report.

| | | |
|---|---|----------------------|
| <u>Designs wastewater load</u> | 4 Bedroom dwelling | 900 L/day |
| <u>Soils characteristics</u> | <u>Horizon A</u> | <u>Horizon B</u> |
| Soil category | 3b Loam | 3b Loam |
| Indicative permeability | 0.5-1.5 m/d | 0.5-1.5 m/d |
| <u>Critical site features</u> | <ul style="list-style-type: none"> Proposed small lot size. Existing dam. One Mile Creek close to west boundary. | |
| <u>Minimum treatment requirements</u> | Primary | |
| <u>Disposal system</u> | <u>Suitability</u> | <u>Area required</u> |
| Absorption trenches | Suitable - Restricted | 150 |
| Subsurface Irrigation | Suitable | 300 m ² |
| ETA Beds | Suitable | 50 m ² |
| Mound | Suitable | 120 m ² |
| <u>Wastewater can be sustainably disposed to land</u> | | Yes |
| <u>Minimum wastewater envelop per lot</u> | | 750 m ² |

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2. INTRODUCTION:

A.C. Geotechnical Pty Ltd (AC) have been engaged to undertake a Land Capability Assessment (LCA) for the proposed subdivision of the approximately 5.67 ha site at 3 Draper Road, Heyfield.

The objectives of the assessment was to determine the following:

- Sub-surface ground profile and geological setting.
- The depth to groundwater (if encountered).
- The permeability of the soil profile.
- The capability of the proposed lots to sustainably manage wastewater within the allotment boundaries.
- A minimum wastewater envelope size for each proposed lot.
- A general management program for each proposed lot that should be put into place to minimise health and environmental impacts of on-site wastewater management, including the impact on surface water and groundwater.

2.1 Proposed Development:

It is proposed to subdivide the site into 13 lots with sizes ranging from 4000 m² m to 4048 m².

For the purpose of this assessment, a design wastewater load for a four (4) bedroom dwelling without water saving fixtures has been used to calculate the minimum required disposal reserve for each proposed lot.

3. SITE DESCRIPTION:

3.1 Site Location:

The subject site is located on the north-east corner of Draper Road and Heyfield-Seaton Road. The site is surrounded by similar size properties, the assumed land use of these properties is summarised in **Table 3.1**.

Table 3.1 -Surrounding land use

| | |
|-------|-------------------------|
| North | Low density residential |
| South | Rural living |
| East | Rural living |
| West | Rural living |

3.2 Site Topography and Condition:

The site is relatively level. A dam is located in the north-west corner and an existing dwelling in the south-east corner. It is understood that the existing dam will be converted to a retarding basin for stormwater management.

Vegetation on the site comprises open turf with native trees around the perimeter.

Site photographs are included in **Appendix B**.

3.3 Key Site Information:

A summary of site characteristic and wastewater loading are included in **Table 3.3**.

Table 3.3 -Key site features

| | |
|--|---|
| Site Address | 3 Draper Road, Heyfield (Lot 1 PS344819) |
| Owner/Applicant | ██████████ |
| Local Council | Wellington |
| Zoning | Low Density Residential (LDRZ) |
| Total Land Area | Proposed lots of approximately 4000 m ² |
| Domestic Water Supply | Reticulated/Tank |
| Anticipated wastewater loads (Litres/day) | <u>EPA Code of practice - onsite wastewater management (2016)</u> Household without water reduction fixtures: 180 L / person / day. Persons = no. bedrooms + 1 (4 + 1 = 5 persons) Design wastewater load 5 x 180 = 900 L / day |
| Organic Material Loading Design Rates | <u>EPA Code of practice - onsite wastewater management (2016)</u> 60 g per person per day (5 x 60) = 300 g/day |
| Availability of sewer | Sewer is not likely to become available to this area in the near future |
| Groundwater Quality | Groundwater is classified as Potable (500-1000 mg/L TDS) www.vvg.org.au |
| Water Table | Local registered bores in the area suggest the ground water is held approximately 20 m below the surface |
| Climate | Average annual rainfall 630.0 mm |
| Flood Potential | Negligible |
| Water catchment area | N/A |
| Proximity to waterways | Onsite dam – neighboring waterway |
| Vegetation | Pasture grasses, scattered native trees |
| Exposure | Generally open |
| Slope | Relatively level |
| Landform | Plains |
| Erosion Potential | Negligible |
| Surface Drainage | Good |
| Rocks and Rock Outcrop | None |

3.4 Site Geology:

According to the Geological Survey of Victoria, the site is in an area of Quaternary aged alluvial deposits, an extract from GeoVic 3 is included in **Figure 3.4**.

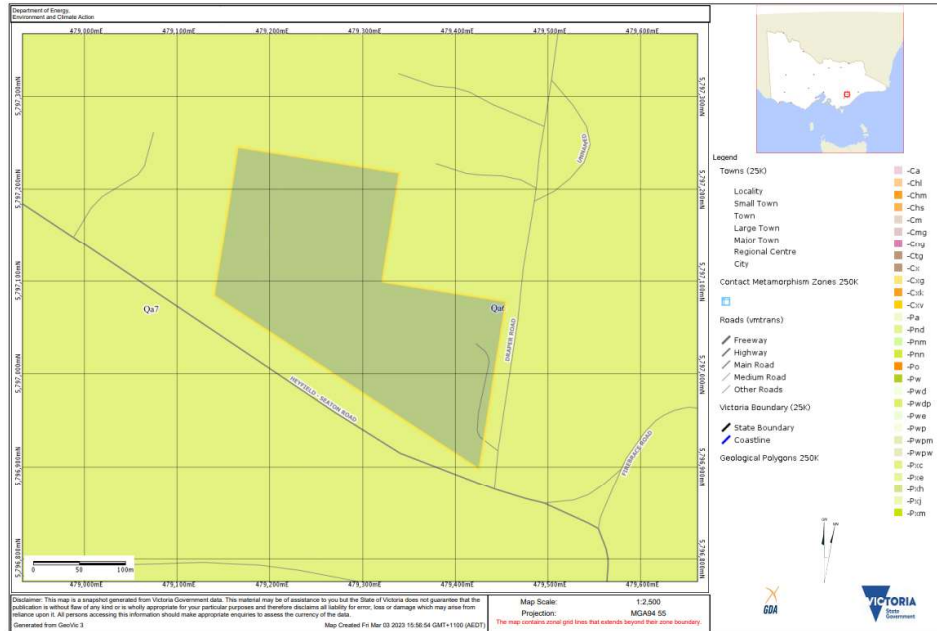


Figure 3.4 Extract of Geological from GeoVic 3

4. SOIL ASSESSMENT AND CONSTRAINTS:

4.1 Soil Profile:

The soil profile encountered during the investigation consisted of pale brown sandy silt overlaying pale brown silty sand.

No groundwater was encountered during this investigation.

No abnormal moisture conditions were identified through this assessment.

Borelogs are included in **Appendix C**.

4.2 Site Exposure:

A general assessment of the site exposure is as follows:

The site is exposed to the prevailing winds. The proposed effluent disposal areas are generally exposed to sun and wind all year round.

4.3 Soil Assessment:

Laboratory analysis on each sample collected included the following:

- Texture Analysis using ribboning technique.
- Modified Emerson Analysis.
- Electrical Conductivity.
- pH analysis.

A summary of the analysis is included in **Table 4.3A & 4.3B**

Table 4.3A -Summary of soil assessment

| BORE HOLE 1 | SAMPLE DEPTH: 200mm | | SAMPLE DEPTH: 600mm | |
|--|-------------------------------|----------|-------------------------------|-----------|
| <u>SOIL ASSESSMENT</u> <u>(AS1547-2012)</u> | <u>SOIL HORIZON: A</u> | | <u>SOIL HORIZON: B</u> | |
| Soil Colour | Pale brown | | Pale brown | |
| Soil Texture | Sandy loam | | Clay loam | |
| Coarse Fragments (%) | None | | None | |
| Soil Structure | Massive | | Weak | |
| Soil Dispersion | Non-dispersive | | Minor dispersion | |
| Soil Permeability | 1.4-3.0 mm/d | | 1.4-3.0 mm/d | |
| Soil Category | 2b | | 3b | |
| Design Irrigation Rate / Design Loading Rate | DIR | 5.0 mm/d | DLR | 15.0 mm/d |
| pH 1:5 Ratio Electronic Method | 5.72 | | 5.02 | |
| Electrical Conductivity | 0.062 dS/m | | 0.079 dS/m | |
| Salinity Hazard | Non-saline | | Non-saline | |

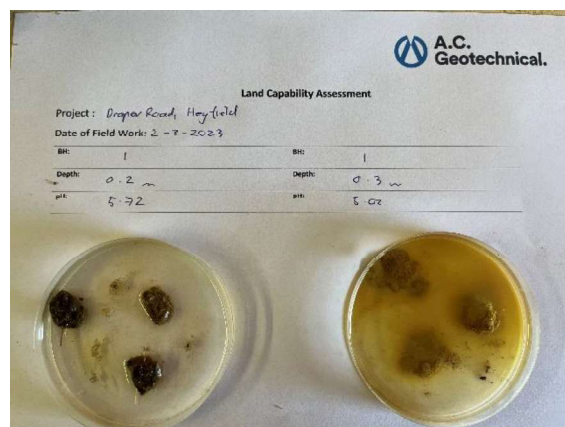


Figure 4.3A Laboratory Analysis

Table 4.3B -Summary of soil assessment

| BORE HOLE 6 | SAMPLE DEPTH: 200mm | | SAMPLE DEPTH: 600mm | |
|--|------------------------|----------|------------------------|----------|
| <u>SOIL ASSESSMENT</u> <u>(AS1547-2012)</u> | <u>SOIL HORIZON: A</u> | | <u>SOIL HORIZON: B</u> | |
| Soil Colour | Pale brown | | Pale brown | |
| Soil Texture | Sandy loam | | Clay loam | |
| Coarse Fragments (%) | None | | None | |
| Soil Structure | Massive | | Weak | |
| Soil Dispersion | Non-dispersive | | Minor dispersion | |
| Soil Permeability | 1.4-3.0 mm/d | | 1.4-3.0 mm/d | |
| Soil Category | 2b | | 3b | |
| Design Irrigation Rate / Design Loading Rate | DIR | 5.0 mm/d | DIR | 5.0 mm/d |
| pH 1:5 Ratio Electronic Method | 5.61 | | 6.08 | |
| Electrical Conductivity | 0.068 dS/m | | 0.073 dS/m | |
| Salinity Hazard | Non-saline | | Non-saline | |

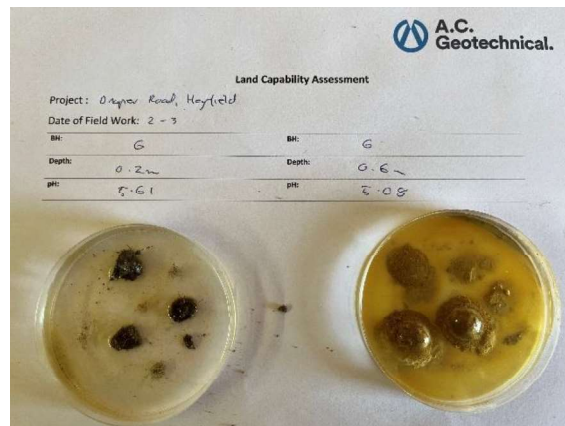


Figure 4.3B Laboratory Analysis

4.4 Field Assessed Permeability:

In situ permeability testing with a constant head permeameter was undertaken in multiple locations across the site, see site plan for locations in **Attachment A**, in accordance with AS 1547-2012 using the constant-head test method. The field assessed permeability was calculated using the Talsma-Hallam constantly maintained head of water equation identified in AS 1547-2012.

$$K_{sat} = \frac{4.4 Q [0.5 \sinh^{-1}(H/2r) - \sqrt{\{(r/H)^2 + 0.25\}} + r/H]}{2\pi H^2}$$

Where:

K_{sat} = saturated hydraulic conductivity of the soil in cm/min.

4.4 = correction factor for a systematic under-estimate of soil permeability in the mathematical derivation of the equation.

Q = rate of loss of water from the reservoir in cm³/min.

H = depth of water in the test hole in cm.

r = radius of the test hole in cm.

A summary of permeability results are included in **Table 4.4**. Permeability Calculations are included in **Appendix D**.

Table 4.4 -Summary of insitu permeability

| Constant Head Permeability | |
|--|----------------------------|
| Rate of loss of water from reservoir (Q) | 87.16 cm ² /min |
| Indicative permeability (K_{sat}) | 0.056 m/day |
| Indicative permeability (K_{sat}) | 0.81 m/day |

Note: The results in the table above are based on average readings taken from the test holes.

The corresponding K_{sat} value of 0.8m/day in EPA Onsite Wastewater Management – Code of Practice Publication No. 891.4 July 2016 Appendix A Table 9 is category 3 (loam soil).

4.5 Critical site Features:

The critical site features are:

- Proposed small lot size.
- Existing dam.
- One Mile Creek close to west boundary.

5. LAND CAPABILITY ASSESSMENT MATRIX:

Table 5.1 and **Table 5.2** includes a Land Capability Assessment (LCA) matrix in accordance with EPA Publication 746.1. The LCA has been developed for the whole site however soils information relates to soils within the vicinity of the proposed wastewater envelope.

Table 5.1 -Land capability assessment matrix - Site

| Land Features | Land Capability Class Rating | | | | | Site Rating | Comments | Mitigation |
|--|------------------------------|----------------------------------|------------|--|--------------------------|-------------|---------------------------------|---|
| | Very Good (1) | Good (2) | Fair (3) | Poor (4) | Very Poor (5) | | | |
| General Characteristics | | | | | | | | |
| Site drainage / runoff | No visible signs of dampness | Moist soil but no standing water | | Visible signs of dampness i.e. water tolerant plants | Water ponding on surface | 1 | No abnormal moisture conditions | N/A |
| Runoff | None | Low | Moderate | High | Very High | 4 | Small lot sizes | Minimum setback distances can be achieved |
| Flood / inundation potential (yearly return exceedance) | Never | | < 1 in 100 | >1 in 100 to < 1 in 20 | > 1 in 20 | 1 | Negligible flood potential | N/A |
| Proximity to water courses | > 60 metres | | | < 60 metres | | 4 | Onsite dam and One Mile Creek | Secondary treatment of wastewater required on some lots to achieve required setback distances |
| Slope (%) | 0 - 2 | 2 - 8 | 8 – 12 | 12 – 20 | > 20 | 1 | Relatively level | N/A |

| Landslip | No potential for failure | Low potential for failure | High potential for failure | Present or Past Failure | | | | |
|---|--|---------------------------|-------------------------------------|---------------------------|----------------------------------|---|--|---|
| Groundwater table (m) seasonal watertable depth | >5.0 | 2.5 – 5.0 | 2.0 – 2.5 | 1.5 – 2.0 | <1.5 | 1 | Groundwater held at approximately 20 m below the surface | N/A |
| Rock Outcrops (% of land surface containing rocks >200mm) | 0% | <10% | 10-20% | 20-50% | >50% | 1 | None encountered | N/A |
| Erosion Potential | No erosion potential | Minor | Moderate | High | Severe erosion potential | 1 | Negligible | Maintain current level of surface cover where practical |
| Exposure | High sun and wind exposure | | Moderate | Low sun and wind exposure | | 1 | High exposure to sun and wind | N/A |
| Landform | Hill crests, convex side slopes and plains | | Concave side slopes and foot slopes | | Floodplains and incised channels | 1 | Plains | N/A |
| Vegetation Type (land application area) | Turf or pasture | | | | Dense Forest | 1 | Pasture | N/A |
| Fill | No Fill present | | Fill Present | | | 1 | No fill encountered | N/A |
| Rainfall (mm/yr)² | <450 | 450 - 650 | 650 – 750 | 750 - 1000 | >1000 | 2 | Average annual rainfall of 630 mm | LAA size to be determined by water balance calculations |
| Pan evaporation (mm/yr)³ | >1500 | 1250 - 1500 | 1000 – 1250 | - | <1000 | 2 | Annual evaporation of 1290.9 mm | LAA size to be determined by water balance calculations |

Table 5.2 -Land capability assessment matrix - Soils

| Soil Profile Characteristics | | | | | | | | |
|---|------------|------------------------|------------------------|-------------------|----------------|---|---------------------------|---|
| Profile depth | >2.0m | 1.5–2.0m | - | 1.0–1.5m | <1.0m | 1 | Deep soil profile | N/A |
| Shrinkage* (%) | Low <4% | Moderate 4-12% | High 12-20% | Very High >20% | | 1 | Low plasticity sand soils | N/A |
| Permeability* (m/d) | 0.15–0.30 | 0.08–0.15 0.30-0.60 | 0.06-0.08 0.60-1.50 | - 1.50-2.00 | <0.06 >2.00 | 3 | Loam soils | LAA size to be determined by water balance calculations |
| Soil Permeability Category¹ | 2 and 3 | 4 | | 5 | 1 and 6 | 1 | Loam soils | LAA size to be determined by water balance calculations |
| Coarse fragments* (%) | <10 | 10-20 | 20-40 | | >40 | 1 | <10% | N/A |
| Emerson Test* (dispersion / slaking) | 4,6,8 | 5 | 7 | 2,3 | 1 | 7 | Slaking/no dispersion | N/A |
| Electrical Conductivity (Ece) (dS/m) | <0.3 | 0.3-0.8 | 0.8-2.0 | 2.0-4.0 | >4.0 | 1 | Non-saline | N/A |
| pH | 6-8 | | 4.5-6 | | <4.5, >8 | 3 | Slightly acidic | N/A |

¹ Source: AS1547-2012

² Source BOM station – Glenmaggie Weir (085034)

³ Source BOM station – East Sale Airport (085314) 2019

* Relevant to soil layer(s) associated with wastewater application

6. MANAGEMENT PROGRAM:

The onsite wastewater system design and management program must suit the capability of the site and will consider the proposed development. The following sections discuss the inputs used to assess the suitability and requirements of EPA approved land based systems. Detailed design for the system is beyond the scope of this study.

6.1 Treatment System:

Primary treatment of all wastewater is considered suitable for a majority of proposed lots, but may be heavily restricted on some lots, depending on site features and proposed developments. Proposed lots 9, 10 and 11 will require secondary treated wastewater disposal to achieve the minimum setback distances to the existing dam/proposed retarding basin. However some land application methods require secondary treatment of wastewater to operate sustainably and efficiently.

Untreated domestic wastewater typically has values of 200-300mg/L biochemical oxygen demand (BOD5) and 200-300mg/L total suspended solids (TSS). Indicative target effluent quality for secondary treatment systems are < 20mg/L BOD5, < 30mg/L TSS and <10cfu/100mL E.Coli.

If secondary treatment of wastewater is required at this site, the two most common options capable of achieving the desired performance are, aerated wastewater treatment systems (AWTS) and single pass sand filters. A summary of these systems is outlined below.

6.1.1 Aerated Wastewater Treatment System (AWTS):

AWTS are pre-fabricated or pre-engineered treatment systems designed to treat small wastewater flows. They are tank-based systems that typically employ the following processes:

- Settling of solids and flotation of scum in an anaerobic primary chamber.
- Oxidation and consumption of organic matter through aerobic biological processes.
- Clarification – secondary settling of solids; and
- Disinfection prior to disposal.

Good maintenance of AWTS (e.g. removal of sludge) is essential to ensure a consistently high level of performance. By law, AWTS are required to be serviced quarterly by an approved maintenance contractor.

6.1.2 Sand Filters:

Sand filters provide advanced secondary treatment to water that has already undergone primary treatment in a septic tank or similar device. They contain approximately 600mm depth of filter media (usually medium to coarse sand, but other media can be incorporated) within a lined excavation containing an underdrain system. Selection of the filter media is critical, and a carefully designed distribution network is necessary. A dosing well and pump is normally used to allow periodic dosing. Depending on the desired level of treatment, sand filters can be single pass or may incorporate partial recirculation.

6.2 Treatment System Location:

Based on requirements of EPA 891.4, above-ground and in-ground treatment systems must comply with the same setback distances to building footings and boundary fences as land application systems.

6.2.1 Septic Tank Sizing:

The minimum septic tank size should be 3,000 L.

6.3 Land Application:

A range of possible land application systems have been considered, such as absorption trenches/beds, evapotranspiration/absorption (ETA) beds, mound systems and sub-surface irrigation. AS1547:2012 outlines factors affecting the construction and operation of common land application systems and a guide to selecting a system taking into consideration site features, subsurface soil conditions and identified constraints. The suitability of EPA approved land based systems are discussed in **Table 6.3**.

Table 6.3 Land Application System

| Land Application | Description | Site Suitability |
|-------------------------------|--|--|
| Absorption Trenches | Trenches are the most common type of land application system and are generally used on lots which are reasonably flat and where water soaks into the soil readily in all weather conditions. Commonly, distribution pipes, self-supporting arch trenching or box trenching are laid in trenches filled with aggregate/rock. Effluent then soaks into the surrounding soil. | Suitable but may be heavy restricted due to site features and proposed developments. Not suitable on proposed lots 9, 10 and 11 |
| ETA Beds | Beds are shallower forms of trenches. Because beds have smaller sidewall area compared with trenches, the absorption provided by sidewall loading is reduced. This is compensated for by reducing the design loading rate. | Suitable |
| Mound System | A mound system permits the absorption area to be sited in a location where the natural water table or impermeable rock approaches the ground surface. The mound is filled with medium-grade sand to provide suitable filtering before intercepting the natural soils. A pump/siphon dosing system distributes effluent uniformly through a bed of aggregate placed at the top of the mound. The sand media in the mound system acts as a secondary treatment system, removing the need for a separate sand filter or AWTS | Suitable |
| Sub-surface Irrigation | Subsurface drip irrigation requires secondary treated effluent dosing lines buried in the topsoil at shallow depth. Irrigation systems operate by both soil absorption and evapotranspiration from plants/trees | Suitable |

6.3.1 Disposal systems:

Water balance modelling has been undertaken to calculate the minimum size of the LAA. The water balance takes into account the average annual rainfall, evaporation data, the daily effluent load, the design irrigation/loading rates for secondary treated effluent, the seasonal crop factor and the retained rainfall. The water balance model is designed so that the land application area is based upon a depth of saturated soil (i.e. water stored within indicative soil porosity) that meets the upper limits of acceptance for each land application method. The water balance must ensure that the soil can sustain growth during the summer months. The design system parameters used for the water balance calculations are summarised in **Table 6.3.1**.

Table 6.3.1 Design System Parameter

| Treatment system | Application System | DIR / DLR | Runoff coefficient | Maximum storage depth |
|---------------------|------------------------|-----------|--------------------|-----------------------|
| Primary treatment | Absorption trenches | 10 | | |
| Secondary treatment | ETA Beds | 20 | 25% | 0 mm |
| | Mound System* | 16 | 25% | 0 mm |
| | Sub-surface irrigation | 4 | 25% | 0 mm |

* Mound disposal system incorporates a secondary treatment sand media, removing the requirement for a separate secondary treatment system

6.4 Land Application Outputs:

Minimum Land Application Area (LAA) sizing for each application method was calculated using water balance calculations, with the exception of absorption trenches. LAA sizing calculations are included in **Appendix D**. The minimum required disposal area for each system is summarised in **Table 6.4**.

Table 6.4 Required Land Application Area (LAA)

| Disposal system | Minimum reserve size required |
|-----------------------|-------------------------------|
| Wastewater output | 900 L / day |
| Absorption trenches | 150 m (1.0 m wide trenches) |
| Subsurface irrigation | 300 m ² |
| ETA Beds | 50 m ² |
| Mound | 120 m ² |

6.5 Proposed Wastewater envelope:

It is recommended that a wastewater envelope with a minimum size of 750 m² be included on each lot. A wastewater envelope of this size will allow adequate space to locate septic tanks/treatment plants and provide multiple disposal options.

6.6 Existing dam:

It is understood that the existing dam in the north-west corner will be converted to a retarding basin for stormwater management on the site. Proposed lots 9,10 and 11 will require disposal of secondary treated wastewater to achieve the required setback distances. Minimum setback for secondary treated wastewater disposal from waterways is 30 m.

6.7 Suitability of proposed lots

The proposed subdivision plan is considered suitable for the sustainable disposal of wastewater to be contained within each lot boundary. A summary of each lot's constraints are included in **Table 6.7**.

Table 6.7 Suitability of proposed lots

| Lot No. | Suitability | Minimum wastewater treatment quality | Comments |
|---------|-------------|--------------------------------------|---|
| 1 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 2 | Suitable | Primary | Lot with existing dwelling |
| 3 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 4 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 5 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 6 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 7 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 8 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 9 | Suitable | Secondary | Heavily restricted due to existing dam/proposed retarding basin |
| 10 | Suitable | Secondary | Heavily restricted due to existing dam/proposed retarding basin |
| 11 | Suitable | Secondary | Heavily restricted due to existing dam/proposed retarding basin |
| 12 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |
| 13 | Suitable | Primary | Potentially heavily restricted due to proposed development – secondary treated will reduce restrictions |

6.8 Designated Area:

The Land Application Area (LAA) shall be located in a designated area to enhance evapotranspiration and shall:

- Not be used for purposes that compromise the effectiveness of the system or access for maintenance.
- Be used only for effluent application.
- Have boundaries clearly delineated by appropriate vegetation or other type of border.
- Have no run-off seepage or effluent beyond the designated area.

The site plan in **Appendix A** presents several potential areas suitable for LAA placement as well as setback areas from site features which must be maintained. Please note that the final LAA

placement is the responsibility of the owner and should be included in a detailed design providing the minimum LAA and setback distances are maintained.

The required LAA will be smaller than that marked on the site plan. An appropriately sized LAA, as discussed in **Section 6.4**, must be located entirely within the area nominated on the site plan.

Setback distances for primary and secondary treated wastewater disposal are included in **Section 6.8.1**.

6.8.1 Setback Distances:

The minimum setback distances for primary and secondary treated wastewater are summarised in **Table 6.8.1**. The proposed LAA must adhere to these minimum setback distances.

Table 6.8.1 Minimum Setback Distances

| Landscape feature or structure | Setback distance (m) (primary treated wastewater) | Setback distance (m) (secondary treated wastewater) |
|---|---|---|
| <u>Building</u> | | |
| Wastewater field up-slope of building | 6 | 3 |
| Wastewater field down-slope of building | 3 | 1.5 |
| Wastewater field up-slope of cutting/escarpment | 30 | 15 |
| <u>Allotment boundary</u> | | |
| Wastewater field up-slope of Allotment boundary | 6 | 3 |
| Wastewater field down-slope of Allotment boundary | 3 | 1.5 |
| <u>Services</u> | | |
| Water supply pipe | 3 | 1.5 |
| Wastewater field up-slope of potable supply channel | 300 | 150 |
| Wastewater field down-slope of potable supply channel | 20 | 10 |
| Gas supply pipe | 3 | 1.5 |
| In-ground water tank | 15 | 7.5 |
| Stormwater drain | 6 | 3 |
| <u>Recreational areas</u> | | |
| Children's grassed playground | 6 | 3 |
| In-ground swimming pool | 6 | 3 |
| <u>Surface water – up-slope of</u> | | |
| Waterway, non-potable creeks, dams, channels | 60 | 30 |
| <u>Groundwater bores</u> | | |
| Category 2b to 6 soils | 40 | 20 |

6.9 Monitoring, Operation and Maintenance:

The septic tank is de-sludged every 3 years; however, this frequency may vary depending on the following conditions.

- whether the tank is an adequate size for the daily wastewater flow
- the composition of the household and personal care products
- the amount of organic matter, fat, oil and grease washed down the sinks
- the use of harsh chemicals such as degreasers
- overuse of disinfectants and bleaches
- the use of antibiotics and other drugs, especially dialysis and chemotherapy drugs
- whether any plastic or other non-organic items are flushed into the tank.

After pump-out, tanks must not be washed out or disinfected. They should be refilled with water to reduce odours and ensure stability of plumbing fixtures. A small residue of sludge will always remain and will assist in the immediate re-establishment of bacterial action in the tank.

To ensure the treatment systems function adequately, residents must:

- Use soapy water (made from natural unscented soap), vinegar and water or bi-carbonate of soda and water to clean toilets and other water fixtures and fittings.
- Read labels to learn which bathroom and laundry products are suitable for septic tanks. Generally plain, noncoloured, unscented and unbleached products will contribute to a well-functioning septic tank.
- Use detergents with low levels of salts (e.g. liquid detergents), sodium absorption ratio, phosphorus and chlorine (see www.lanfaxlabs.com.au).
- Wipe oils and fats off plates and saucepans with a paper towel and dispose of in the kitchen compost bin.
- Use a sink strainer to restrict food scraps entering the septic system.
- Ensure no structures such as pavements, driveways, patios, sheds or playgrounds are constructed over the tank or absorption trench area.
- Ensure the absorption trench area is not disturbed by vehicles or machinery.
- Engage a service technician to check the sludge and scum levels, pumps and alarms annually.
- Keep a record of the location of the tank and the trenches and all maintenance reports (including the dates of tank pump-outs, tank inspections and access openings) and ensure the service technician sends a copy of the maintenance report to the local Council
- Have the tank desludged when the combined depth of the scum and sludge is equal to the depth of the middle-clarified layer.

Indications of failing septic tanks and soil absorption trenches

- Seepage along effluent absorption trench lines in the soil.
- Lush green growth down-slope of the soil absorption trench lines.
- Lush green growth down-slope of the septic tank.
- Inspection pits and/or the soil absorption trenches consistently exhibiting high water levels.
- Soil absorption trench lines become waterlogged after storms.
- General waterlogging around the land disposal area.

- Presence of dead and dying vegetation (often native vegetation) around and down-slope of the land disposal areas.
- A noxious odour near the tank and the land disposal area.
- Blocked water fixtures inside the house, with sewage overflowing from the relief point.
- High sludge levels within the primary tank (within about 150 mm of inlet pipe).
- Flow obstructed and not able to pass the baffle in the tank.
- The scum layer blocking the effluent outflow.

6.9.1 Storm Water Management:

All stormwater must be disposed of to the legal point of discharge, or as per drainage design specification.

If stormwater is disposed onsite, it should be disposed downslope of the LAA.

Note: An agricultural drain (AG) must be installed on the high side of the wastewater envelope. The drain is to be installed a minimum of 100mm into the naturally occurring clay soils and allow sufficient fall to intercept and drain all overland and subsurface run-off to a legal point of discharge. If a legal point of discharge cannot be obtained, the drainage line may discharge directly to the surface soils, a minimum distance of 10 metres beyond the wastewater disposal area.

7. CONCLUSIONS:

From this investigation it is concluded that the use of an on-site wastewater treatment and disposal system is environmentally sustainable on each proposed lot, if the recommendations made in this report are followed.

8. REFERENCES:

- Environmental Protection Authority – Guidelines for Environmental Management Code of Practice – Onsite Wastewater Management, July 2016 ~ Publication 891.4
- Municipal Association Victoria (MAV) January 2014, Model Land Capability Assessment Framework
- Australian/New Zealand Standard AS/NZS 1547-2012 – On-site domestic wastewater management.
- A.C. Geotechnical Pty Ltd - Field and Laboratory data (where applicable) collected and recorded.
- Environmental Protection Authority - “Code of Practice - Septic Tanks”, March 1996” ~ Publication 451.
- Environmental Protection Authority, Information Bulletin- “Land Capability Assessment for onsite Domestic Wastewater Management”, March 2003 ~ Publication 746.1.

Notes

1. Wastewater envelopes must be setback a minimum of 3.0 on the low side and 6.0 m on the high side of from all boundaries.
2. Wastewater envelopes area must be setback a minimum of 3.0 m from the low side and 6.0 m from the high side of the proposed building envelopes.
3. 2. Wastewater envelopes on Lot 9-13 must be setback a minimum of 30 m from any water body (onsite retarding Basin, One Mile Creek).
4. Minimum setback distances are outlined in **Section 6.8.1**.
5. The proposed wastewater envelope will be smaller than the indicated suitable area on this plan.



Development Concept Plan with Aerial Image
Sheet 1 of 1



Not to Scale
Investigation locations are approximate

Legend

- Investigation Location.
- Wastewater envelope to be located within this suitable area.

Attachment A: Site Plan 23037 001

3 Draper Road
Heyfield
Date of field work: 2 March 2023





Development Concept Plan
with Aerial Image
Sheet 1 of 1



Not to Scale
Investigation locations are approximate

Legend

-  Investigation Location.
-  Wastewater envelope to be located within this suitable area.

Attachment A: Site Plan 23037 001

3 Draper Road
Heyfield
Date of field work: 2 March 2023

Appendix B

Site Photographs



Appendix C

Borelog

Borehole Record BH01



| Project Number | 23037_001 | Date | 2/03/2023 |
|----------------|--|-----------------|--------------------------|
| Project | Land Capability Assessment | Drilling Method | HA |
| Location | 3 Draper Road, Heyfield | Logged | AC |
| Depth (m) | Description | | |
| 0.00 | Sandy SILT (ML): Pale brown, firm, dry-moist. | | |
| | | | Disturbed sample - 0.2 m |
| 0.40 | Silty SAND (SW): Pale brown, fine to coarse grain, well graded, medium dense, moist, minor clay content. | | |
| | | | Disturbed sample - 0.6 m |
| 2.00 | Borehole terminated - target depth achieved | | |

Borehole Record BH02

| Project Number | 23037_001 | Date | 2/03/2023 |
|------------------|--|-----------------|-----------|
| Project Location | Land Capability Assessment 3 Draper Road, Heyfield | Drilling Method | HA |
| | | Logged | AC |
| Depth (m) | Description | | |
| 0.00 | Sandy SILT (ML): Pale brown, firm, dry-moist. | | |
| 0.30 | Silty SAND (SW): Pale brown, fine to coarse grain, well graded, medium dense, moist, minor clay content. | | |
| 2.00 | Borehole terminated - target depth achieved | | |

Borehole Record BH03

| Project Number | 23037_001 | Date | 2/03/2023 |
|----------------|--|-----------------|-----------|
| Project | Land Capability Assessment | Drilling Method | HA |
| Location | 3 Draper Road, Heyfield | Logged | AC |
| Depth (m) | Description | | |
| 0.00 | Sandy SILT (ML): Pale brown, firm, dry-moist. | | |
| 0.35 | Silty SAND (SW): Pale brown, fine to coarse grain, well graded, medium dense, moist, minor clay content. | | |
| 2.00 | Borehole terminated - target depth achieved | | |

Borehole Record BH04

| Project Number | 23037_001 | Date | 2/03/2023 |
|----------------|--|-----------------|-----------|
| Project | Land Capability Assessment | Drilling Method | HA |
| Location | 3 Draper Road, Heyfield | Logged | AC |
| Depth (m) | Description | | |
| 0.00 | Sandy SILT (ML): Pale brown, firm, dry-moist. | | |
| 0.30 | Silty SAND (SW): Pale brown, fine to coarse grain, well graded, medium dense, moist, minor clay content. | | |
| 2.00 | Borehole terminated - target depth achieved | | |

Borehole Record BH05

| Project Number | 23037_001 | Date | 2/03/2023 |
|------------------|--|-----------------|-----------|
| Project Location | Land Capability Assessment 3 Draper Road, Heyfield | Drilling Method | HA |
| | | Logged | AC |
| Depth (m) | Description | | |
| 0.00 | Sandy SILT (ML): Pale brown, firm, dry-moist. | | |
| 0.20 | Silty SAND (SW): Pale brown, fine to coarse grain, well graded, medium dense, moist, minor clay content. | | |
| 2.00 | Borehole terminated - target depth achieved | | |

Borehole Record BH06



| Project Number | 23037_001 | Date | 2/03/2023 |
|------------------|--|-----------------|--------------------------|
| Project Location | Land Capability Assessment 3 Draper Road, Heyfield | Drilling Method | HA |
| | | Logged | AC |
| Depth (m) | Description | | |
| 0.00 | Sandy SILT (ML): Pale brown, firm, dry-moist. | | |
| | | | Disturbed sample - 0.2 m |
| 0.30 | Silty SAND (SW): Pale brown, fine to coarse grain, well graded, medium dense, moist, minor clay content. | | |
| | | | Disturbed sample - 0.6 m |
| 2.00 | Borehole terminated - target depth achieved | | |

Appendix D

Constant Head Calculations & Water Balance

INSITU CONSTANT HEAD PERMEABILITY



| | | | |
|-------------------------|---------------|------------------------|-----------|
| Project Address: | 3 Draper Road | Project Number: | 23037 |
| Location: | Heyfield | Date: | 6/03/2023 |
| Client: | [REDACTED] | | |

INPUT DATA

| | Borehole | | Reservoir |
|---------------------------------|----------|------------------|--------------------------|
| Borehole diameter | 100 cm | Diameter | 97 mm |
| Borehole Depth | 500 cm | Base area | 295.4426 mm ² |
| Water level from surface | 250 cm | | |
| Depth of water in hole | 250 cm | | |

FIELD DATA

| | <u>Test 1</u> | <u>Test 2</u> | <u>Test 3</u> | <u>Test 4</u> | |
|-----------------------------|--------------------------|---------------|---------------|---------------|--------------------|
| Time intervals (min) | Water depth in reservoir | | | | |
| Initial Depth | 200 | 200 | 200 | 200 | |
| 5 | | | | | |
| 10 | | | | | |
| 15 | | | | | |
| 20 | 141 | 136 | 148 | 139 | Average |
| Q (cm ² /min) | 87.155567 | 94.541632 | 76.815076 | 90.109993 | 87.155567 |
| Ksat (cm/min) | 0.056015583 | 0.060762666 | 0.049369666 | 0.057914416 | 0.056015583 |
| Ksat (m/d) | 0.806624389 | 0.874982388 | 0.71092319 | 0.833967588 | 0.806624389 |

ABSORPTION TRENCH SIZE CALCULATIONS



| Project Address: | 3 Draper Road | Project Number: | 23037 |
|-----------------------------------|---------------|------------------------|-----------|
| Location: | Heyfield | Date: | 6/03/2023 |
| Client: | [REDACTED] | | |
| INPUT DATA | | | |
| Daily flow allowance (per person) | 180 L | | |
| Daily wastewater volume | 900 L | | |
| Effluent quality | Primary | | |
| Soil texture | Loam | | |
| Soil structure | Weak | | |
| Soil category | 3b | | |
| Indicative Permeability | 0.5-1.5 Ksat | | |
| Design Loading Rate | 6 mm/d | | |
| ABSORPTION TRENCHES | | | |
| L = Q / (DLR x W) | | | |
| Where: | | | |
| L = length of trench | | | |
| Q = Design daily flow in L/day | | | |
| DLR = Design Loading rate in mm/d | | | |
| W = width of trench in m | | | |
| Width of trench | 0.7 m | Width of trench | 1 m |
| Length = | 214 m | Length = | 150 |

WATER BALANCE SUBSURFACE IRRIGATION



| | | | | | | | | | | | | | | |
|-----------------------------------|----------------------------|-----------------|-----------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------------|
| Project Address: | 3 Draper Road | Project Number: | 23037 | | | | | | | | | | | |
| Location: | Heyfield | Date: | 6/03/2023 | | | | | | | | | | | |
| Client: | [REDACTED] | | | | | | | | | | | | | |
| INPUT DATA | | | | | | | | | | | | | | |
| Daily flow allowance (per person) | 180 L | | | | | | | | | | | | | |
| Daily wastewater volume | 900 L | | | | | | | | | | | | | |
| Effluent quality | Secondary | | | | | | | | | | | | | |
| Effective rainfall | 0.75 % | | | | | | | | | | | | | |
| Soil texture | Loam | | | | | | | | | | | | | |
| Soil structure | Weak | | | | | | | | | | | | | |
| Soil category | 3b | | | | | | | | | | | | | |
| Indicative Permeability | 0.5-1.5 Ksat | | | | | | | | | | | | | |
| SUBSURFACE IRRIGATION | | | | | | | | | | | | | | |
| DLR | 3.5 mm/d | | | | | | | | | | | | | |
| Porosity | 45 % | | | | | | | | | | | | | |
| Maximum Storage Depth | 0 mm | | | | | | | | | | | | | |
| Crop Factor - standard pasture | 0.85 | 0.85 | 0.85 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.85 | 0.85 | 0.85 | | |
| crop factors - Lucene | 0.95 | 0.9 | 0.85 | 0.8 | 0.7 | 0.55 | 0.55 | 0.65 | 0.75 | 0.85 | 0.95 | 1 | | |
| Crop factor - Shade | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | | |
| Crop factor - woodlot | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | | |
| Rainfall Data | Glenmaggie Weir (085034) | | | | | | | | | | | | | |
| Evaporation Data | East Sale Airport (085314) | | | | | | | | | | | | | |
| Parameter | Unit | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec | Total |
| Days in month | | 31 | 28 | 31 | 30 | 31 | 30 | 31 | 31 | 30 | 31 | 30 | 31 | 365 |
| Rainfall (mm) | | 56.7 | 48.5 | 54 | 50.8 | 47.5 | 48.9 | 37.2 | 42.3 | 50.2 | 60.3 | 71.8 | 61.8 | 630 |
| Evaporation (mm) | | 191.6 | 143.8 | 120.1 | 84.2 | 59.7 | 40 | 50.1 | 63.7 | 86.5 | 109.5 | 159.9 | 181.8 | 1290.9 |
| Output | | | | | | | | | | | | | | |
| Evapotranspiration (mm) | | 162.86 | 122.23 | 102.09 | 50.52 | 35.82 | 24 | 30.06 | 38.22 | 51.9 | 93.075 | 135.92 | 154.53 | 1001.2 |
| Percolation (mm) | | 108.5 | 98 | 108.5 | 105 | 108.5 | 105 | 108.5 | 108.5 | 105 | 108.5 | 105 | 108.5 | 1277.5 |
| Total Output (mm) | | 271.36 | 220.23 | 210.59 | 155.52 | 144.32 | 129 | 138.56 | 146.72 | 156.9 | 201.58 | 240.92 | 263.03 | 2278.7 |
| Inputs | | | | | | | | | | | | | | |
| Effective Rainfall (mm) | | 42.525 | 36.375 | 40.5 | 38.1 | 35.625 | 36.675 | 27.9 | 31.725 | 37.65 | 45.225 | 53.85 | 46.35 | 472.5 |
| Application Rate (mm) | | 93 | 84 | 93 | 90 | 93 | 90 | 93 | 93 | 90 | 93 | 90 | 93 | 1095 |
| Total Inputs (mm) | | 135.53 | -220.2 | 133.5 | 128.1 | 128.63 | 126.68 | 120.9 | 124.73 | 127.65 | 138.23 | 143.85 | 139.35 | 1567.5 |
| Storage Calculations | | | | | | | | | | | | | | |
| Waste Loading (mm) | | 228.84 | 183.86 | 170.09 | 117.42 | 108.7 | 92.325 | 110.66 | 115 | 119.25 | 156.35 | 187.07 | 216.68 | |
| Volume of Wastewater (mm) | | 27900 | 25200 | 27900 | 27000 | 27900 | 27000 | 27900 | 27900 | 27000 | 27900 | 27000 | 27900 | 328500 |
| Cumulative Storage (mm) | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Land area required | | | | | | | | | | | | | | 300 m2 |

NUTRIENT BALANCE



| | | | |
|-----------------------------------|---------------|------------------------|-----------|
| Project Address: | 3 Draper Road | Project Number: | 23037 |
| Location: | Heyfield | Date: | 6/03/2023 |
| Client: | [REDACTED] | | |
| Nitrogen Balance -Nitrogen | | | |
| Hydraulic Loading | 900 | l/day | |
| Effluent N concentration | 25 | mg/l | |
| Daily N loading | 22500 | mg/day | |
| Annual N loading | 8212500 | mg/year | |
| Denitrification loss | 20 | % | |
| Denitrification loss | 6570000 | mg/year | |
| Total annual N loading | 6.57 | kg/year | |
| Plant uptake | 220 | kg/ha/year | |
| Minimum area for uptake | 299 | m ² | |

Appendix E

Property Reports

PROPERTY DETAILS

Address: **3 DRAPER ROAD HEYFIELD 3858**

Lot and Plan Number: **Lot 1 PS344819**

Standard Parcel Identifier (SPI): **1\PS344819**

Local Government Area (Council): **WELLINGTON**

Council Property Number: **326157**

Directory Reference: **Vicroads 695 L9**

www.wellington.vic.gov.au

SITE DIMENSIONS

All dimensions and areas are approximate. They may not agree with those shown on a title or plan.



Area: 56685 sq. m (5.67 ha)

Perimeter: 1121 m

For this property:

— Site boundaries

— Road frontages

Dimensions for individual parcels require a separate search, but dimensions for individual units are generally not available.

Calculating the area from the dimensions shown may give a different value to the area shown above

For more accurate dimensions get copy of plan at [Title and Property Certificates](#)

UTILITIES

Rural Water Corporation: **Southern Rural Water**

Urban Water Corporation: **Gippsland Water**

Melbourne Water: **Outside drainage boundary**

Power Distributor: **AUSNET**

STATE ELECTORATES

Legislative Council: **EASTERN VICTORIA**

Legislative Assembly: **GIPPSLAND EAST**

PLANNING INFORMATION

Property Planning details have been removed from the Property Reports to address duplication with the Planning Property Reports which are DELWP's authoritative source for all Property Planning information.

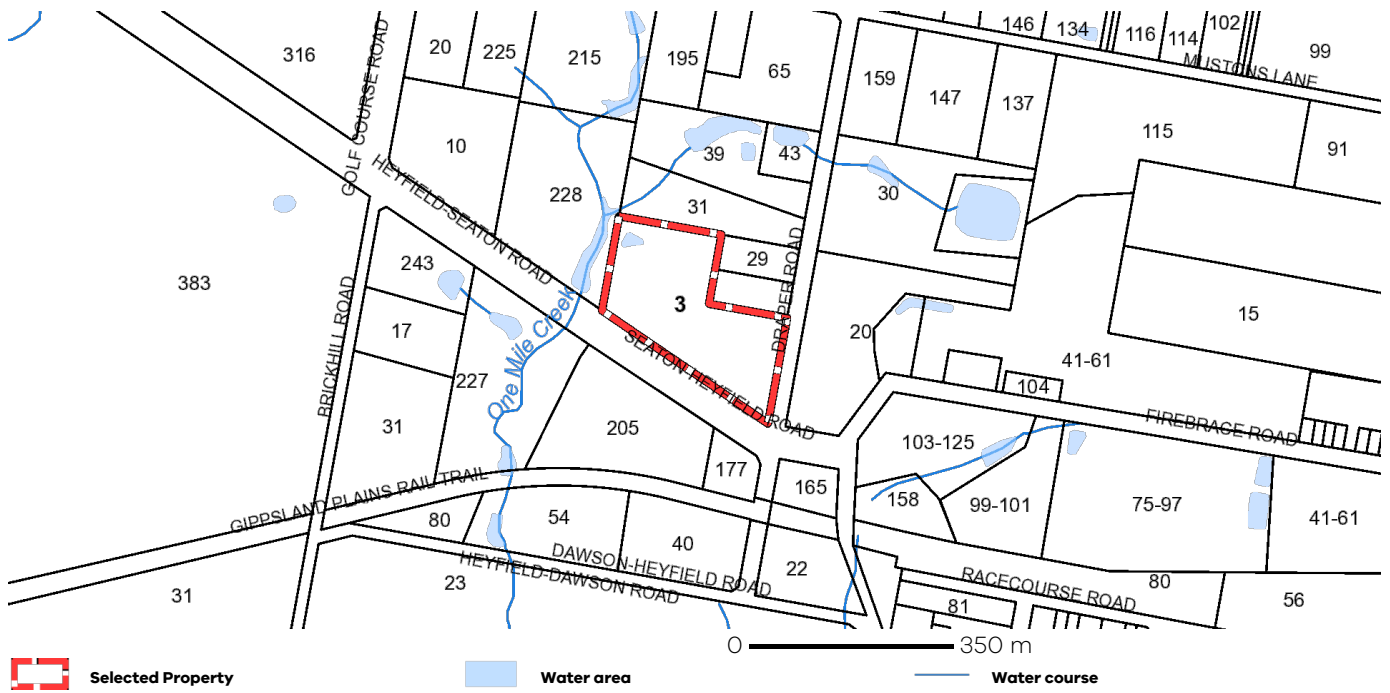
The Planning Property Report for this property can found here - [Planning Property Report](#)

Planning Property Reports can be found via these two links

Vicplan <https://mapshare.vic.gov.au/vicplan/>

Property and parcel search <https://www.land.vic.gov.au/property-and-parcel-search>

Area Map



From www.planning.vic.gov.au at 03 March 2023 03:46 PM

PROPERTY DETAILS

Address: **3 DRAPER ROAD HEYFIELD 3858**

Lot and Plan Number: **Lot 1 PS344819**

Standard Parcel Identifier (SPI): **1\PS344819**

Local Government Area (Council): **WELLINGTON**

Council Property Number: **326157**

Planning Scheme: **Wellington**

Directory Reference: **Vicroads 695 L9**

www.wellington.vic.gov.au

[Planning Scheme - Wellington](#)

UTILITIES

Rural Water Corporation: **Southern Rural Water**

Urban Water Corporation: **Gippsland Water**

Melbourne Water: **Outside drainage boundary**

Power Distributor: **AUSNET**

STATE ELECTORATES

Legislative Council: **EASTERN VICTORIA**

Legislative Assembly: **GIPPSLAND EAST**

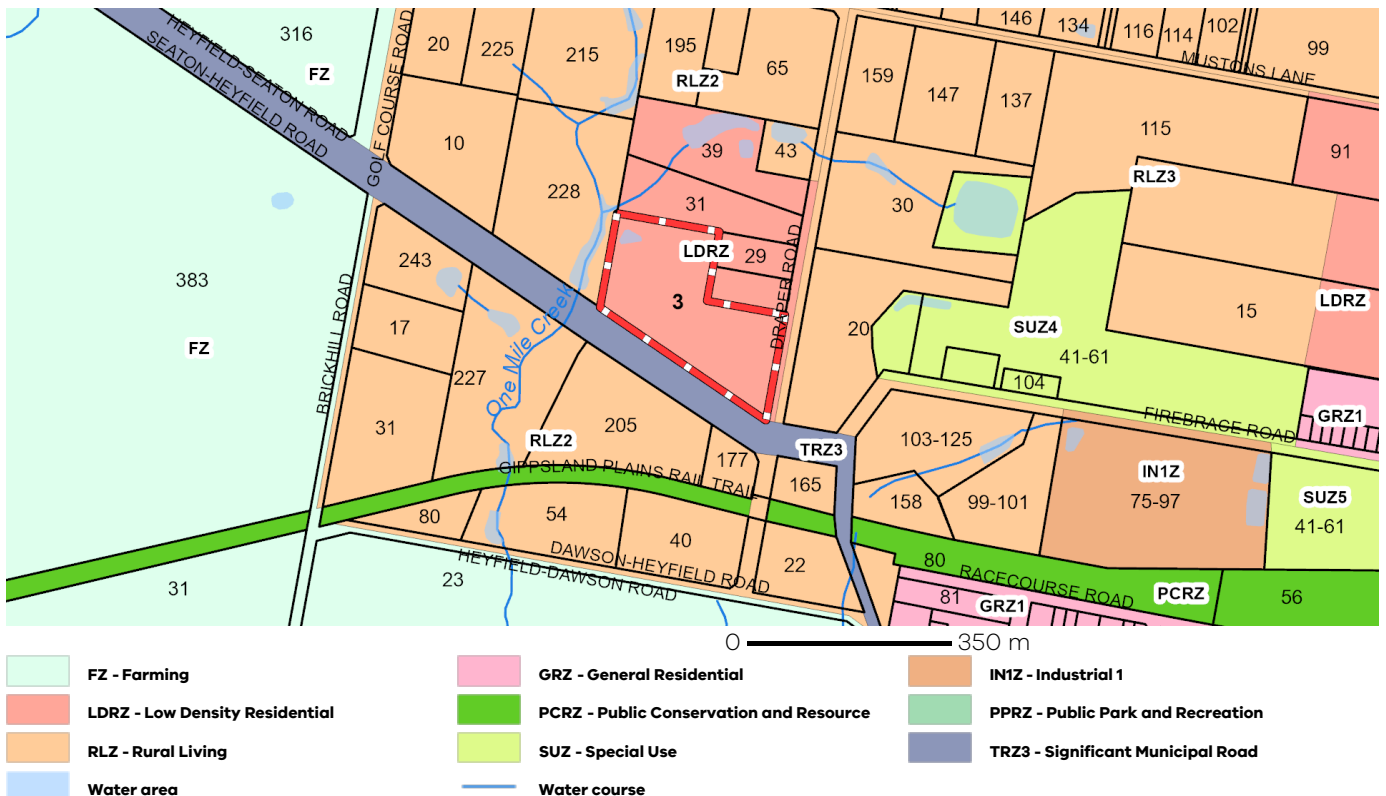
OTHER

Registered Aboriginal Party: **Gunaikurnai Land and Waters
Aboriginal Corporation**

[View location in VicPlan](#)

Planning Zones

[LOW DENSITY RESIDENTIAL ZONE \(LDRZ\)](#)
[SCHEDULE TO THE LOW DENSITY RESIDENTIAL ZONE \(LDRZ\)](#)



Note: labels for zones may appear outside the actual zone - please compare the labels with the legend.

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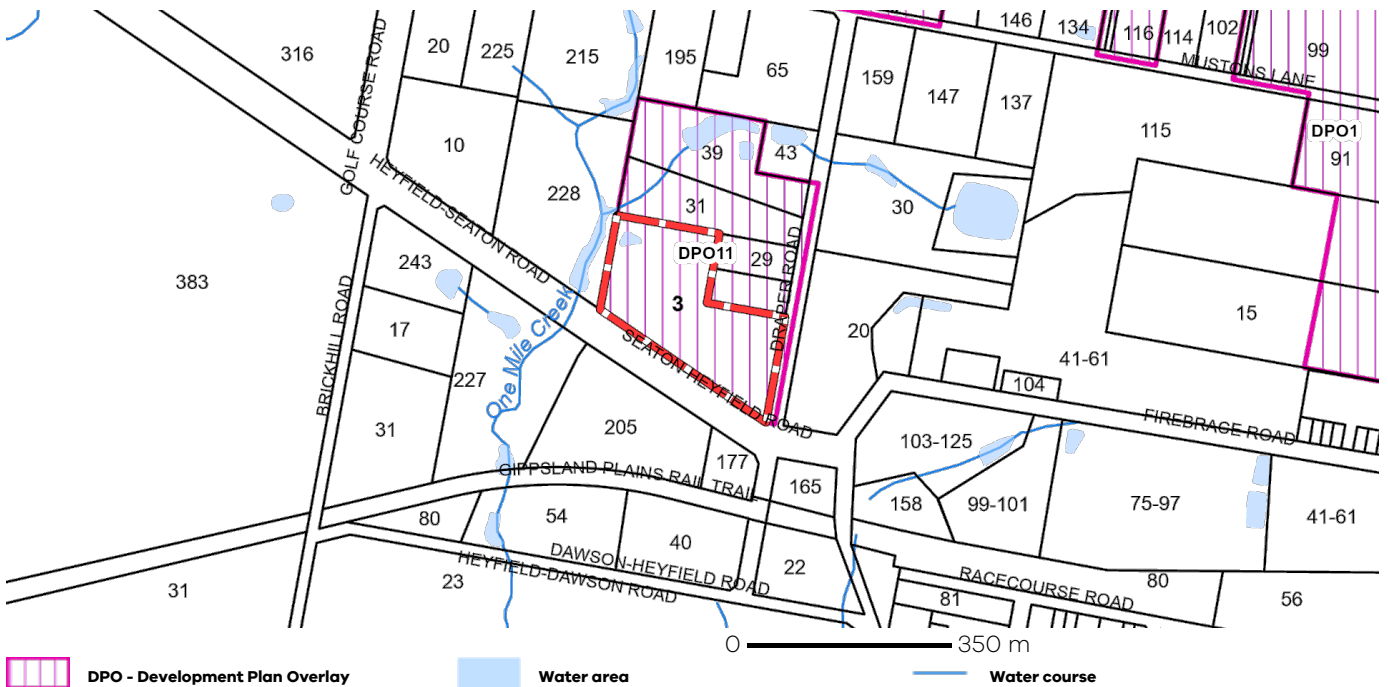
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Notwithstanding this disclaimer, a vendor may rely on the information in this report for the purpose of a statement that land is in a bushfire prone area as required by section 32C (b) of the Sale of Land 1962 (Vic).

Planning Overlays

[DEVELOPMENT PLAN OVERLAY \(DPO\)](#)

[DEVELOPMENT PLAN OVERLAY - SCHEDULE 11 \(DPO11\)](#)



Note: due to overlaps, some overlays may not be visible, and some colours may not match those in the legend

OTHER OVERLAYS

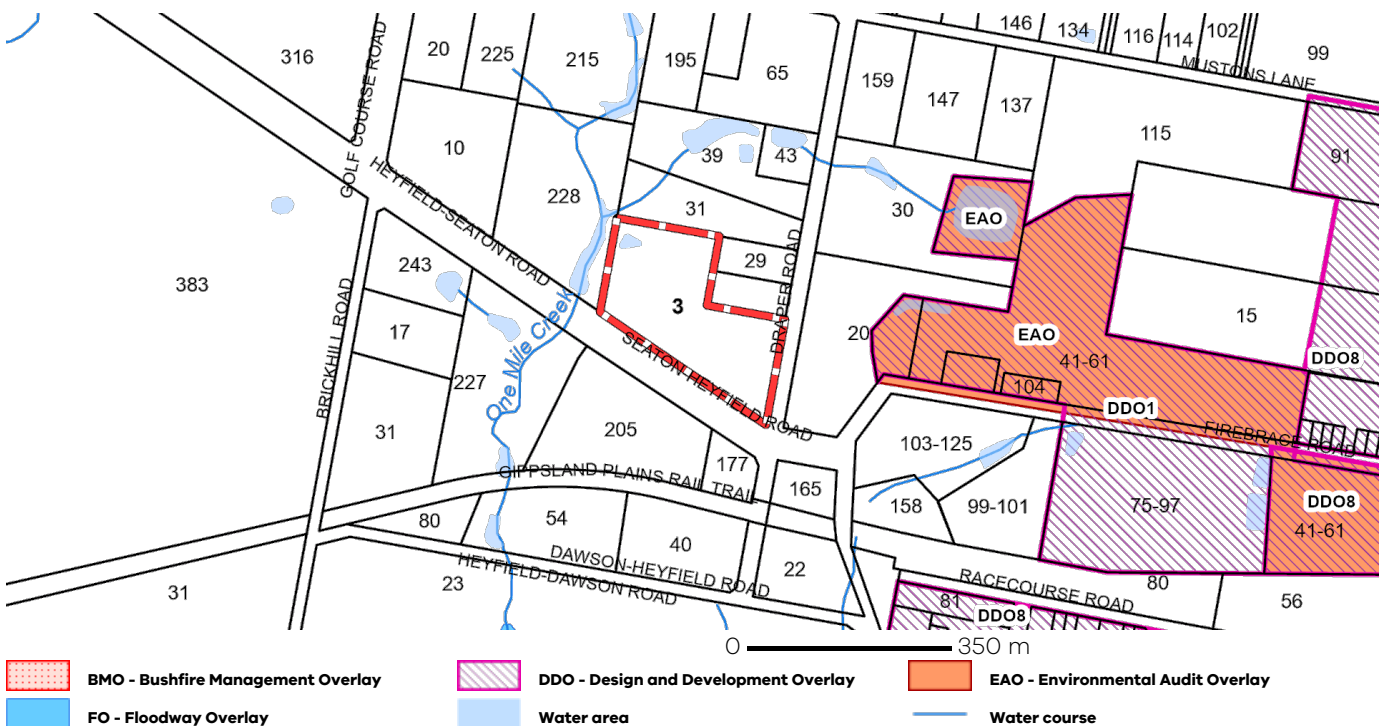
Other overlays in the vicinity not directly affecting this land

[BUSHFIRE MANAGEMENT OVERLAY \(BMO\)](#)

[DESIGN AND DEVELOPMENT OVERLAY \(DDO\)](#)

[ENVIRONMENTAL AUDIT OVERLAY \(EAO\)](#)

[FLOODWAY OVERLAY \(FO\)](#)



Note: due to overlaps, some overlays may not be visible, and some colours may not match those in the legend

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Notwithstanding this disclaimer, a vendor may rely on the information in this report for the purpose of a statement that land is in a bushfire prone area as required by section 32C (b) of the Sale of Land 1962 (Vic).

Areas of Aboriginal Cultural Heritage Sensitivity

All or part of this property is an 'area of cultural heritage sensitivity'.

'Areas of cultural heritage sensitivity' are defined under the Aboriginal Heritage Regulations 2018, and include registered Aboriginal cultural heritage places and land form types that are generally regarded as more likely to contain Aboriginal cultural heritage.

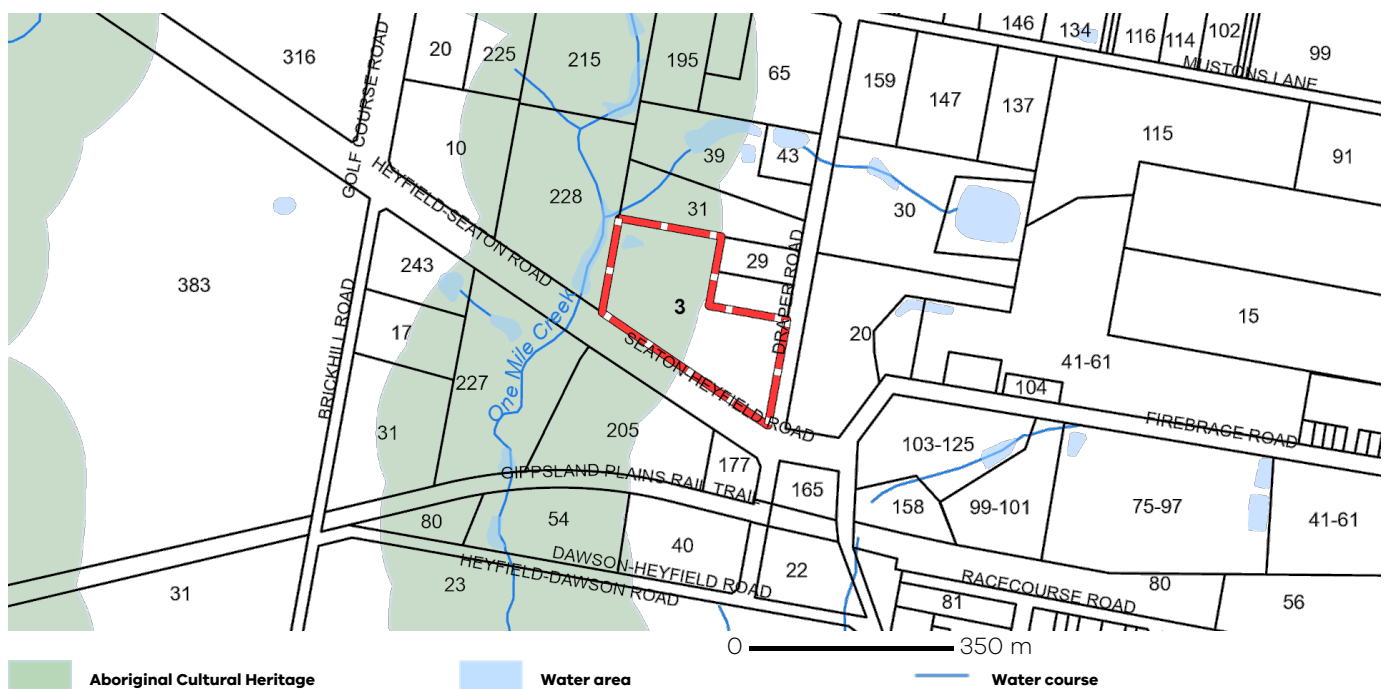
Under the Aboriginal Heritage Regulations 2018, 'areas of cultural heritage sensitivity' are one part of a two part trigger which require a 'cultural heritage management plan' be prepared where a listed 'high impact activity' is proposed.

If a significant land use change is proposed (for example, a subdivision into 3 or more lots), a cultural heritage management plan may be triggered. One or two dwellings, works ancillary to a dwelling, services to a dwelling, alteration of buildings and minor works are examples of works exempt from this requirement.

Under the Aboriginal Heritage Act 2006, where a cultural heritage management plan is required, planning permits, licences and work authorities cannot be issued unless the cultural heritage management plan has been approved for the activity.

For further information about whether a Cultural Heritage Management Plan is required go to <http://www.aav.nrms.net.au/aavQuestion1.aspx>

More information, including links to both the Aboriginal Heritage Act 2006 and the Aboriginal Heritage Regulations 2018, can also be found here - <https://www.aboriginalvictoria.vic.gov.au/aboriginal-heritage-legislation>



Further Planning Information

Planning scheme data last updated on 1 March 2023.

A **planning scheme** sets out policies and requirements for the use, development and protection of land.

This report provides information about the zone and overlay provisions that apply to the selected land.

Information about the State and local policy, particular, general and operational provisions of the local planning scheme that may affect the use of this land can be obtained by contacting the local council

or by visiting <https://www.planning.vic.gov.au>

This report is NOT a **Planning Certificate** issued pursuant to Section 199 of the **Planning and Environment Act 1987**.

It does not include information about exhibited planning scheme amendments, or zonings that may affect the land.

To obtain a Planning Certificate go to Titles and Property Certificates at Landata - <https://www.landata.vic.gov.au>

For details of surrounding properties, use this service to get the Reports for properties of interest.

To view planning zones, overlay and heritage information in an interactive format visit

<https://mapshare.maps.vic.gov.au/vicplan>

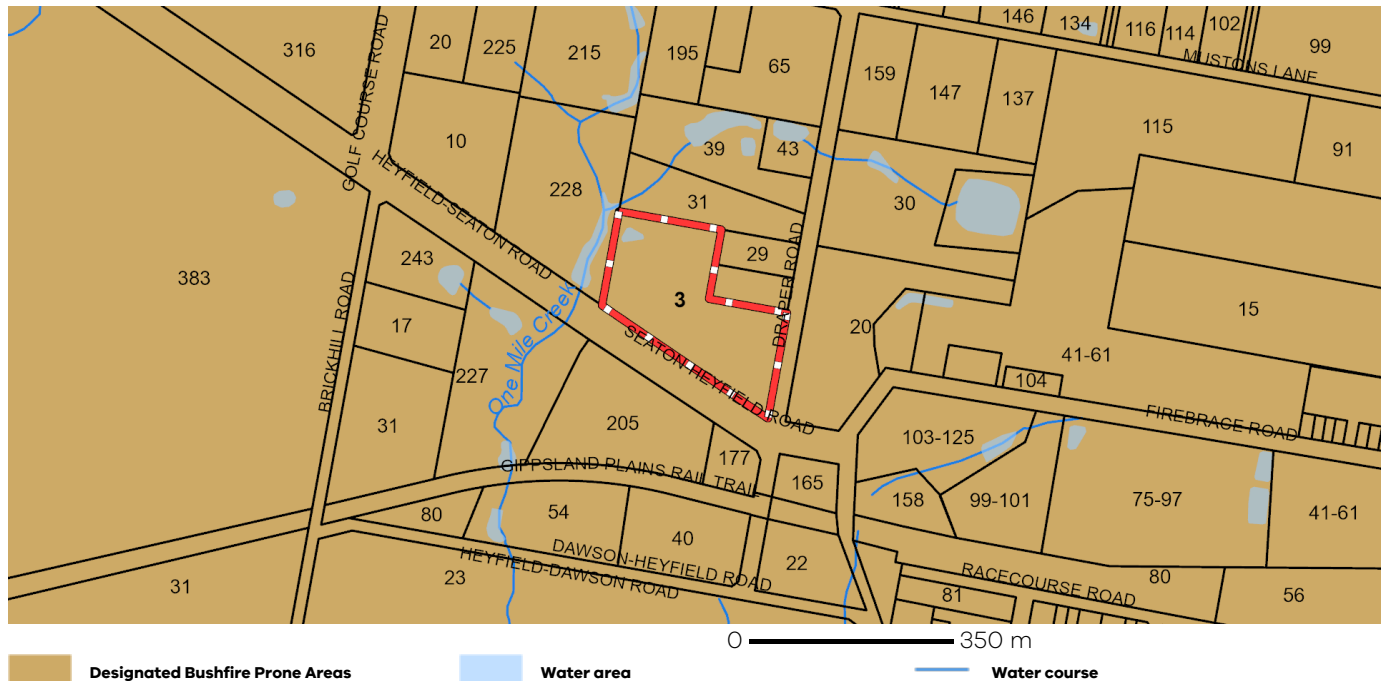
For other information about planning in Victoria visit <https://www.planning.vic.gov.au>

Designated Bushfire Prone Areas

This property is in a designated bushfire prone area. Special bushfire construction requirements apply to the part of the property mapped as a designated bushfire prone area (BPA). Planning provisions may apply.

Where part of the property is mapped as BPA, if no part of the building envelope or footprint falls within the BPA area, the BPA construction requirements do not apply.

Note: the relevant building surveyor determines the need for compliance with the bushfire construction requirements.



Designated BPA are determined by the Minister for Planning following a detailed review process. The Building Regulations 2018, through adoption of the Building Code of Australia, apply bushfire protection standards for building works in designated BPA.

Designated BPA maps can be viewed on VicPlan at <https://mapshare.vic.gov.au/vicplan/> or at the relevant local council.

Create a BPA definition plan in [VicPlan](#) to measure the BPA.

Information for lot owners building in the BPA is available at <https://www.planning.vic.gov.au>.

Further information about the building control system and building in bushfire prone areas can be found on the Victorian Building Authority website <https://www.vba.vic.gov.au>. Copies of the Building Act and Building Regulations are available from <http://www.legislation.vic.gov.au>. For Planning Scheme Provisions in bushfire areas visit <https://www.planning.vic.gov.au>.

Native Vegetation

Native plants that are indigenous to the region and important for biodiversity might be present on this property. This could include trees, shrubs, herbs, grasses or aquatic plants. There are a range of regulations that may apply including need to obtain a planning permit under Clause 52.17 of the local planning scheme. For more information see [Native Vegetation \(Clause 52.17\)](#) with local variations in [Native Vegetation \(Clause 52.17\) Schedule](#)

To help identify native vegetation on this property and the application of Clause 52.17 please visit the Native Vegetation Information Management system <https://nvim.delwp.vic.gov.au/> and [Native vegetation \(environment.vic.gov.au\)](#) or please contact your relevant council.

You can find out more about the natural values on your property through NatureKit [NatureKit \(environment.vic.gov.au\)](#)